MEMORANDUM



| TO: | Michael Harney - Mayor | FROM: | Nedal Barbar/Chad Newton |
|-----|---|--------------|--------------------------|
| | Rudolph Liebenberg -CAO | PROJECT No.: | 180347100 |
| RE: | Sandy Beach Existing Lagoon Summary & Recommended Upgrade Options | DATE: | 11/17/2020 |

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Morrison Hershfield was retained by the Summer Village of Sandy Beach to evaluate the performance of the existing lagoon and propose recommended options for improvement. Background information was reviewed, and the lagoon currently exceeds its capacity.

This memorandum summarizes existing conditions and proposed options for improvement to the evaporation lagoon onsite.

Available Information

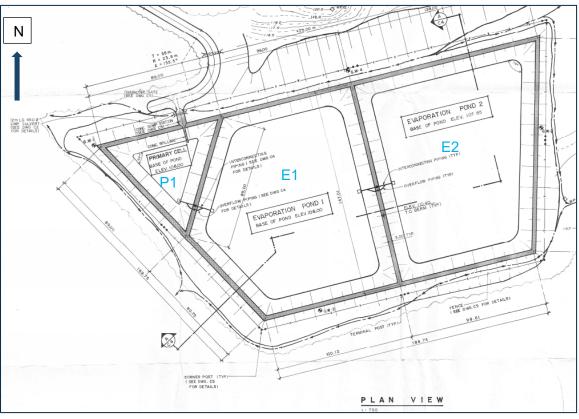
The following information was reviewed. No other data or reports were available.

- Project Binder supplied by the Summer Village
- Regional Lagoon Feasibility Study by Wardrop Alberta LTD, dated July 1990
- As-built drawings by Maxim Engineering, dated July 29, 1991
- Parts 3 and 4 of the *Standards and Guidelines for Municipal Waterworks, Wastewater and Storm Drainage Systems* (March 2013)

Existing System Conditions

The existing lagoon was constructed in Year 1991 and consists of three cells, a primary cell and two evaporation cells. Some of the berms are in need of repair and therefore the sewage level in lagoon will need to be lowered to accommodate the repairs.

Figure 1.0 shows a site plan of the area.



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Figure 1.0

Table 1.0 shows the dimensions and volumes for all cells at maximum operation level but excludes
 freeboard. Original design included a Free Board (FB) of 0.9 m but currently the lagoon has a remaining freeboard of 0.1 m, therefore exceeding its capacity and occupying the freeboard area.

| Lagoon Cell | Treatment Cell | Side Slopes (H:V) | Max. Operation Liquid Depth | Liquid Volume m ³ | Liquid Surface Area m ² | 0.9 m Depth Freeboard Volume m ³ | Total Volume (includes FB) m ³ |
|----------------|------------------|-------------------------|-----------------------------------|------------------------------------|---|--|---|
| P1 | Primary Cell | 4:1 | 1.50 | 2,460 | 2,100 | 1,890 | 4,350 |
| E1 | Evaporation Cell | 4:1 | 1.50 | 20,660 | 15,200 | 13,680 | 34,340 |
| E2 | Evaporation Cell | 4:1 | 1.65 | 20,020 | 14,850 | 13,365 | 33,385 |
| Total | | | | 43,140 | 32,150 | 28,935 | 72,075 |

| Table 1.0: Existing Lagoon Cells | Table 1.0: | Existing | Lagoon | Cells |
|----------------------------------|------------|----------|--------|-------|
|----------------------------------|------------|----------|--------|-------|

The lagoons are designed to hold a liquid volume of approx. 43,140 m³ (without the freeboard volume), however, the liquid is currently at approx. 68,860 m³ with approximately 0.1 m of freeboard remaining. The total available volume in existing lagoon is 72,075 m³, including the freeboard (28,935 m³).

This indicates that the freeboard depth is currently at 0.8 m and 89% of the design freeboard is occupied as shown in Table 2.0 below:



| | Depth (m) | Approx. Volume (m³) | % of Total FB Volume |
|--------------------|-----------|------------------------|-------------------------|
| Design Freeboard | 0.9 | 28,935 | n/a |
| Occupied Freeboard | 0.8 | 25,720 | 89% |
| Vacant Freeboard | 0.1 | 3,215 | 11% |

Table 2.0: Existing Freeboard

Approximately 25,720 m³ of volume is in excess of the design capacity and needs to be relocated elsewhere through hauling, pumping, or a possible new lagoon cell in order to keep the existing lagoon running at its optimum operating capacity.

The Regional Lagoon Feasibility Study by Wardrop indicated that there will be a time in the design life of the lagoon where inflow will exceed evaporation. The lagoon level will continue to rise at that point until storage capacity is has been exceeded. The study stated a major discharge would be required to reduce the storage volume.

It appears this lagoon has passed that point and is currently reaching near the top of berm.

Existing Lagoon Limitations

At the time of design, the evaporation lagoon was designed based on a net evaporation rate of 180 mm/yr. This rate was confirmed to be near the calculated average based on available climate data from years 1980 to 2010. The existing cells were assessed to understand the sewage inflow limits. **Table 3.0** below shows that an influent of 15.9 m³/day (5787 m³/year) will evaporate from lagoon and not cause further increase in levels and will ensure the 0.9 m freeboard is available.

| Table 3.0: Influent Limitat | | iy Layoon |
|---------------------------------------|---------|-----------|
| Influent Flow | m3/day | 15.9 |
| | m³/year | 5,787 |
| Net Evaporation Rate | mm/yr | 180 |
| | | |
| Primary Cells | | |
| Influent | m³/yr | 5,787 |
| Net Evaporative Water Loss in Lagoons | m³/yr | 378 |
| Effluent to Evaporation Cell #1 | m³/yr | 5,409 |
| | | |
| Evaporation Cell #1 | | |
| Influent from Primary Cell | m³/yr | 5,409 |
| Net Evaporative Water Loss in Lagoons | m³/yr | 2,736 |
| Effluent to Evaporation Cell #2 | m³/yr | 2,673 |
| | | |
| Evaporation Cell #2 | | |
| Influent from Evap. Cell #1 | m³/yr | 2,673 |
| Net Evaporative Water Loss in Lagoons | m³/yr | 2,673 |
| Excess Effluent | m³/yr | - |

Table 3.0: Influent Limitations of Existing Lagoon



Based on conversations with the Summer Village, there was approximately 10,500 m³/year (28.7 m³/day) being discharged into the lagoon, which is greater than the limit of 5787 m³/year (15.9 m³/day) available thru evaporation. Therefore, there is an excess of 4713 m³ being dumped into the lagoon per year which explains why the freeboard is currently at 0.1 m versus 0.9 m. Based on the occupied freeboard volume of 25,720 m³, **it may have taken an estimate of 6-7 years for lagoon freeboard to fill to that level based on the excess rate of 4713 m³/year.**

It should also be noted that in 2009 it was recommended that a desludging of the lagoon should be completed. However, this recommendation was not completed, and the capacity of the lagoon was further reduced by the sludge volume.

Design Standards and Guidelines

According to Parts 3 of the *Standards and Guidelines for Municipal Waterworks, Wastewater and Storm Drainage Systems* (March 2013), Section 3.4.2.1 states the following for evaporation ponds:

- "For systems with average daily design flows of less than 250 m³, the system may be designed with one evaporation cell. Provision must be made at the inlet to the cell for settlement and removal of sludge."
- "In no case shall an evaporation lagoon provide less than 3 years of storage capacity based on average daily design flows."
- "Evaporation cells shall not have a depth greater than 1.5 m."

One of the evaporation cells, Cell E2, is a little over the depth limit but still acceptable (1.65 m vs 1.5 m). The minimum volume required based on a 3-year storage capacity and a daily average sewage flow of 26 m³/d (Year 2016) would be approximately 28,000 m³. The current lagoon volume exceeds this amount, however, the design should consider the rate of evaporation in sizing the surface area of the lagoon.

Recommended Options for Improving Lagoon Operation

Based on the existing conditions onsite, we have proposed 4 options for improving the function of this existing lagoon. **All options will require desludging**. A discharge extension which has been requested by MH to Alberta Environment would be required to bring the sewage level down further so repairs can be made. All options are discussed below:

- Option 1: Discharge existing lagoon to a water level to allow repair to berms. Approval by Alberta Environment will be required prior to discharge. Restrict the capacity of the facility to 15.9 m³/day (5,787 m³/yr) and haul the rest 12.9 m³/day (4713 m³/yr) to another lagoon nearby. Long term hauling contract with Onoway or another facility would be required.
- Option 2: Upgrade the system to 30 m³/day (10,950 m³/yr) by constructing another evaporation cell with a total surface of area of approximately 29,000 m². A sketch of Option 2 is shown below in Figure 2.0.



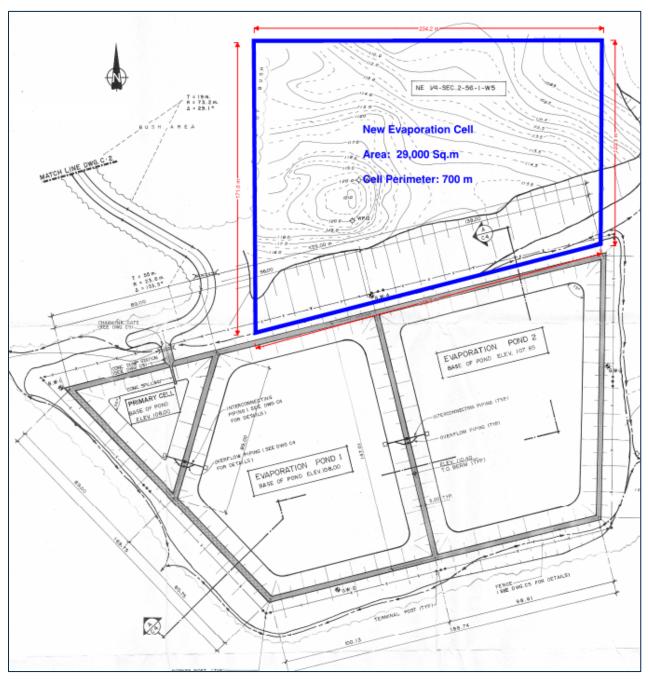


Figure 2.0

3. **Option 3**: Construct a conventional lagoon that consists of a primary cell with a storage volume of 2,700 m³ (includes freeboard) and storage cell with a volume of 13,000 m³ (includes freeboard) based on the inflow of 28.7 m³/day (10,500 m³/year). The storage cell will need to be deeper at 3.0 m with an outlet that is directed to Bard Lake. This option is subject to approval by Alberta Environment and the First Nation Reserve neighboring Bard Lake. A sketch of Option 3 is shown below in **Figure 3.0**. Decommissioning of the existing evaporation pond may be required.



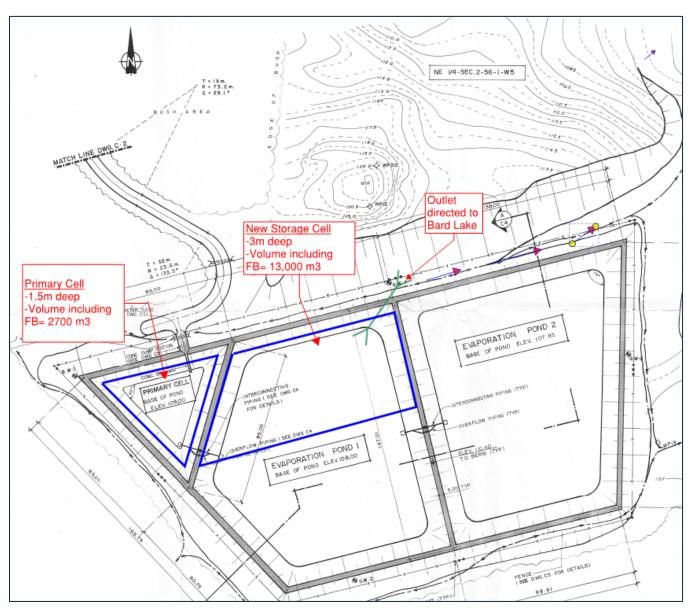


Figure 3.0

4. **Option 4**: Leave Lagoon operational but not in operation. Discharge existing lagoon to a water level to allow repair to berms. Approval by Alberta Environment will be required prior to discharge. Monitoring and reporting will be required to maintain lagoon capacity levels as required by Alberta Environment. Redirect all sewage to another lagoon for a period of time until volume is available in the cells. Hauling contract with Onaway or another facility required.



Costs:

Costs for each option are shown below in Table 4.0. The additional cost for Option 4 is negligible as the only costs that occur are desludging and repair to eroded berms. **Desludging of all cells is** required by all options, and have been included in cost estimate. Berm repair was assumed to be within the freeboard depth of Evaporation Pond 2 only.

| Table 4.0: Cost Estimates for Each Option | | | | |
|---|----|-----------|--|--|
| | Т | otal Cost | | |
| Option 1 - Restrict Inflow to 15.9 m ³ /day* | \$ | 252,000 | | |
| Option 2 - Evaporation Cell | \$ | 2,531,000 | | |
| Option 3 - Conventional Lagoon | \$ | 1,119,000 | | |
| Option 4 – Haul to Another Lagoon Offsite* | \$ | 258,000 | | |

| Table 4.0: Cost | Estimates f | for Fach | Option |
|-----------------|-------------|----------|--------|
| | Louinateo i | | Option |

*Option 1 & 4 require an additional \$4713/year and \$10,500/year, respectively.

Detailed breakdown of this conceptual estimate is provided in Appendix A. Note that Option 1 and Option #4 would require an additional \$4713 per year and \$10,500 per year, respectively, for hauling to Onoway or another facility.

Note that the costs above are within ± 30% error. As design progresses, more accurate cost estimates can be provided.

Funding:

Research was completed on possible grants for water and wastewater systems in Alberta. The following are possible sources of funding for this project:

- 1. Water for Life Program Provides cost-shared funding to regional commissions or groups of 2 or more municipalities and consists of 2 funding streams. One stream for new infrastructure and one for existing infrastructure.
- 2. Alberta Municipal Water/Wastewater Partnership (AMWWP) Provides cost-shared funding to eligible municipalities to help build municipal facilities for water supply and treatment, and wastewater treatment and disposal.

Grant application are due yearly by Nov 30th, 2020. Engineering and repairs should be applicable.

Should you have any questions or concerns, please feel free to contact the Chad Newton or myself.

Yours truly, Morrison Hershfield Limited

Chad Newton, MBA, PMP, Principal PM Department Manager West, Senior Project Manager

Nedal Barbar, P.Eng Project Engineer

Morrison Hershfield Limited APEGA PERMIT TO PRACTICE P2277

Brian Fanson, P.Eng. BFano Nou 18, 2020

Attached: Appendix A: Cost Breakdown



Appendix A - Detailed Cost Breakdown for Each Option

Summary

| | Total Cost |
|---|-----------------|
| Option 1 - Restrict Flow to 15.9 m3/day | \$ 252,134 |
| Option 2 - Evaporation Cell | \$ 2,531,313 |
| Option 3 - Conventional Lagoon | \$ 1,118,572 |
| Option 4 - Haul to Another Lagoon | \$ 257,921 |

| Option - 1 (Restrict to 15.9 m3/day) | | | | | | | | |
|--|----------------|-------|-----------|---------|----------|--|--|--|
| Item | Unit | Qty | Unit Rate | ate Co: | | | | |
| Additonal Delta Hauling Costs | m3 | 4,713 | \$ 1.00 | \$ | 4,713 | | | |
| (additonal cost to haul to Onoway vs Current Lagoon) | | | | | | | | |
| Sludge Removal and Disposal | LS | 1 | \$136,627 | \$ | 136,627 | | | |
| Repair to Berms (assumed just freeboard area of berm) | m ³ | 3187 | \$20 | \$ | 63,748 | | | |
| Pumping between Cells | LS | 1 | \$33,846 | \$ | 33,846 | | | |
| Sewage Sampling | LS | 1 | \$4,400 | \$ | 4,400.00 | | | |
| Enviromental Sampling for Sewage Disposal on Agricultural Land | LS | 1 | \$8,800 | \$ | 8,800.00 | | | |
| | | | Sub Total | \$ | 252,134 | | | |
| | | | | | | | | |
| Total Cost | | | | \$ | 252,134 | | | |

* assumed current rate to orginal lagoon is ~\$4/m3 * new rate assumption \$5/m3

* these costs would be either abosorbed by municipality or past directly onto residents

| Option - 4 (Haul to Another Lagoon Offsite) | | | | | | | | | |
|--|----------------|--------|-----------|-----------|----------|--|--|--|--|
| Item | Unit | Qty | Unit Rate | Cost | | | | | |
| | | | | | | | | | |
| Additonal Delta Hauling Costs | m3 | 10,500 | \$ 1.00 | \$ | 10,500 | | | | |
| (additonal cost to haul to Onoway vs Current Lagoon) | | | | | | | | | |
| Sludge Removal and Disposal | LS | 1 | \$136,627 | \$ | 136,627 | | | | |
| Repair to Berms (assumed just freeboard area of berm) | m ³ | 3187 | \$20 | \$ | 63,748 | | | | |
| Pumping between Cells | LS | 1 | \$33,846 | \$ | 33,846 | | | | |
| Sewage Sampling | LS | 1 | \$4,400 | \$ | 4,400.00 | | | | |
| Enviromental Sampling for Sewage Disposal on Agricultural Land | LS | 1 | \$8,800 | \$ | 8,800.00 | | | | |
| | | | | | | | | | |
| | | | Sub Total | <u>\$</u> | 257,921 | | | | |
| | | | | | | | | | |
| | | | | | | | | | |
| Total Cost | | | | \$ | 257,921 | | | | |

* assumed current rate to orginal lagoon is ~\$4/m3

* new rate assumption \$5/m3

* these costs would be either abosorbed by municpality or past directly onto residents

| Option -3 (Conve | ntional Lagoon) | | | |
|--|-----------------|-------|-----------|-----------------|
| Item | Unit | Qty | Unit Rate | Cost |
| | | | | |
| New Storage and Prima | | i. | | |
| Topsoil and subsoil Stripping 1m deep | m³ | 7405 | \$5 | \$ 37,026 |
| Topsoil Placement (150mm depth) | m ² | 7405 | \$1.5 | \$ 11,108 |
| Pond Storage Excavation | m³ | 6500 | \$10 | \$ 65,000 |
| Clay Bottom Placement (1.0m Thick) | m ³ | 7405 | \$20 | \$ 148,105 |
| Wave protection on side slopes (250mm Thick) | m ² | 594 | \$65 | \$ 38,610 |
| Containment Berms | m ³ | 12882 | \$20 | \$ 257,645 |
| Hydroseed | m ² | 7405 | \$1.0 | \$ 7,405 |
| Sludge Removal and Disposal | LS | 1 | \$136,627 | \$ 136,627 |
| Pumping between Cells | LS | 1 | \$33,846 | \$ 33,846 |
| Sewage Sampling | LS | 1 | \$4,400 | \$ 4,400.00 |
| Enviromental Sampling for Sewage Disposal on Agricultural Land | LS | 1 | \$8,800 | \$ 8,800.00 |
| Control/Overflow Manhole or Chamber | each | 1 | \$35,000 | \$ 35,000 |
| Splash Pad | LS | 2 | \$10,000 | \$ 20,000 |
| Outlet Pipe | LS | 1 | \$25,000 | \$ 25,000 |
| | | | Sub Total | \$ 828,572 |
| | | | | |
| Mob/Demob/Insurances | 10% | | | \$ 82,857 |
| Contingencies and Engineering | 25% | | | \$ 207,143 |
| Total Cost | | | | \$ 1,118,572 |

| Option - 2 (New Evaporation Cell) | | | | | | | | |
|--|----------------|-------|-----------|----|----------|--|--|--|
| Item | Unit | Qty | Unit Rate | | Cost | | | |
| New Evaporatio | n Cell | | | _ | | | | |
| Topsoil and subsoil Stripping 1m deep | m³ | 29000 | \$5 | \$ | 145,000 | | | |
| Topsoil Placement (150mm depth) | m ² | 29000 | \$1.5 | \$ | 43,500 | | | |
| Pond Storage Excavation | m ³ | 40000 | \$10 | \$ | 400,000 | | | |
| Clay Bottom Placement (1.0m Thick) | m ³ | 29000 | \$20 | \$ | 580,000 | | | |
| Wave protection on side slopes (250mm Thick) | m ² | 480 | \$65 | \$ | 31,200 | | | |
| Containment Berms | m ³ | 14969 | \$20 | \$ | 299,376 | | | |
| Hydroseed | m ² | 29000 | \$1.0 | \$ | 29,000 | | | |
| Sludge Removal and Disposal | LS | 1 | \$136,627 | \$ | 136,627 | | | |
| Repair to Berms (assumed just freeboard area of berm) | m ³ | 3187 | \$20 | \$ | 63,748 | | | |
| Pumping between Cells | LS | 1 | \$33,846 | \$ | 33,846 | | | |
| Sewage Sampling | LS | 1 | \$4,400 | \$ | 4,400.00 | | | |
| Enviromental Sampling for Sewage Disposal on Agricultural Land | LS | 1 | \$8,800 | \$ | 8,800.00 | | | |
| Control/Overflow Manhole or Chamber | each | 1 | \$35,000 | \$ | 35,000 | | | |
| Splash Pad | LS | 2 | \$10,000 | \$ | 20,000 | | | |
| Install Fence | m | 495 | \$90 | \$ | 44,550 | | | |